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AN EXPERIMENTAL STUDY ON TUNED LIQUID DAMPER AND MASS DAMPER UNDER DYNAMIC LOADING FOR FRAME STRUCTURES

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ABSTRACT

In this paper, the liquid damper (TLD) and mass damper (TMD) are usedto minimize the structuralretort due to moving action. Several experimental sets are measuredon a model of structure TLDand TMD systems to check their efficiencies under vocal excitation. Rectangular shapes ofTLD with altered type ofliquiddeepness ratios are checked and various frequency ratios are measured. The experimental revision was approved by expending shake table apparatus. The various values are measured using accelerometers and LVDT sensors. Trendysituation of TMD the toughness and restraining are remain constant for experimental study. From this study, it is established that for TLD, as water deepnessraises it leads to minimum amplitude. Thehugecontroller of vibration is noticed under resonant condition by using liquid damper. The outcomes show that TMD can decrease the extreme displacement of assembliesbelow harmonic motions.

Keywords: *Splashing, tuned liquid damper,tuned mass damper, Vocal excitation, Earthquake, Retortcontroller, ANSYS.*

I. INTRODUCTION

Vibration controller is a significant feature when planning buildings, especially if they are high. Buildings can get exposed to considerable shaking owed towardsairstream and earthquakes. When an earthquake waves portableover the building, it is exposed massive forces, acceleration and displacement that make the building extremely unbalanced and ultimately it failures. Mass damper, Liquid dampers, base isolators and other additional restraining structures are amongst the several replacements recycled to decrease the shakings on the assemblies. A TLD is water restricted in a vessel that uses the splashing energy of the liquid to decrease the vibrant reply of the scheme when the system is exposed to excitation. TLD has similarly establishing to be very real in abandoning shakings produced owed to wind.

Altered Liquid Damper (TLD) is an inactive controller device which takes connected in structures to overwhelm straight shakings in the assemblies. TLD is fundamentally a liquid filled tank which is strictly joined to the topmost of the structure. It trusts on the splashing wave emerging and contravention at the allowed surface of the fluid to disintegrate a share of the energy unconfined throughout the vibrant occurrence and therefore raises the correspondent restraining of the assembly. When occurrence of tank gesture is near to the occurrence of the container liquid significance happens. At significance, great quantity of splashing and wave contravention happens at the allowed surface of the fluid which scatters a substantial extent of energy. TLD grants numerous benefits above other restraining schemes such as small installation, running and process rate, rarer motorised difficulties subsequently no moving portions are present-day and can be useful to controller dissimilar shaking kinds of multi-degree of self-determination organizations.

TMD is a sticky spring-mass element, when devoted to an exciting key assembly, delivers an occurrence responsibility hysteresis that rises the restraining in the assembly. The efficiency of TMD for regulatory structural reaction is delicate to its constraints i.e. mass, frequency, and damping ratio. TMD turns as a subordinate exciting structure once joined to main exciting system. When TMD is altered to occurrence near to natural occurrence of assembly, shaking of structure makes TMD to shake in significance, dispersing extreme shaking energy over restraining in obstacle and also due to comparative movement of damper with deference to the structure. In accumulation TMD is good-looking as it disperses a substantiallevel of shaking energy of main structure without necessitating any joining to ground. Numerous TMDs have been positively realized universal for wind reaction controller in structures, funnels and turrets.

To get optimal reaction the normal frequency of the secondary mass is continuously tuned to that of primary structure such that when that specific frequency of the assembly get motivated, the TMD will reverberate out of stage with the structural gesture. The extra expanse of energy constructed up in the structure is converted to the secondary mass and degenerate due to comparative gesture established amongst them at an upcoming phase.

The chief objective of this study was to determine the behaviour of structure when devoted to TLD and TMD to decrease structural reaction under vibrant loading. For TLD numerous water depth and excitation frequency the standards for displacement and acceleration are determined with TLD and without TLD measured. For TMD numerous input frequencies are established for altered mass ratios with TMD and without TMD experimentally.

II. EXPERIMENTAL PROCEDURE

The arrangement of the steel structure model on the shake table along with liquid damper and mass damper is displayed in fig. (1) and fig. (3).in shake table only horizontal motions are going to be carried. The shapeof table is in circular shape such that its diameter is 40 cm and its radius is 20 cm. The weight of the apparatus or machine is 30 kg. The worth offrequency that can be used to study experiment is 10 Hz. The essential excitation occurrence is useful to the assemblyby using microprocessor 3-phase AC system.The TLD model is placed exactly centre of bottom base plate which is existing in figure (1). The material which is used for TLD tanks are of acrylic sheet. The entire mass ratio used for TMD system was around 10 % of the mass of the structure. Severalfigures of TMDs remained used for structural model. The steel model is made with mild steel of sufficient thickness for rigid floor, supported by four steel rods with dimension of 250 × 250 × 270 mm for each floor for all storeys. The columns are jointed to the base plates by welding. The measured parameters are accelerationanddisplacementfor structure.The vertical member column is made with steel rod, and beam acted as story mass, thus the movement is in laterally to longitudinal axis. The models were intended with removable parts and relaxedentrance for energy dissipation devices. The setup for without TLD is as specified in fig. (2).The story masses are same, and they can be adjusted along the elevation of the main columns. The details of liquid damper are specified in table 1.

Table 1: Details of TLD parameters

Shape of TLD	TLD Dimensions		
	Length L(cm)	Width B(cm)	Depth H(cm)
Rectangular	12	8	40

A) Essential Splashing occurrence of the TLD

The Fundamental Sloshing frequency of a TLD.ω_l, can be determined using below equation

$$\omega_l = \frac{1}{2\pi}\sqrt{\frac{\pi g}{2a}\tanh\left(\frac{\pi h}{2a}\right)}$$

Where g = acceleration due to gravity, h = stable water level,l = length of the tank in the direction of sloshing motion.

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Figure 1: Experimental setup for tuned liquid damper

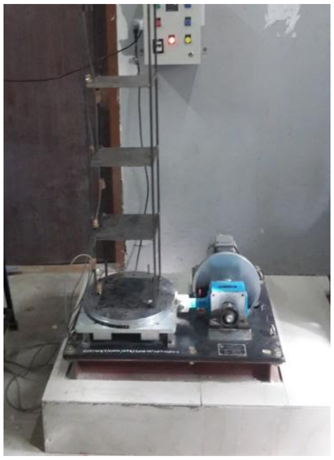

Figure 2: Setup without tuned liquid damper

Figure 3: Structure with tuned mass damper

B) Modelling in ansys

The initial data which has been used for modelling of TLD and TMD are stated following:

a) Structure four-storey rigid jointed simple frame.
b) Materials properties (M20) and Steel (Fe415)
c) Size of column = 0.25 m x 0.45 m
d) Size of beam = 0.25 m x 0.40 m
e) Modulus of elasticity = 2×10^5 KN/m^2

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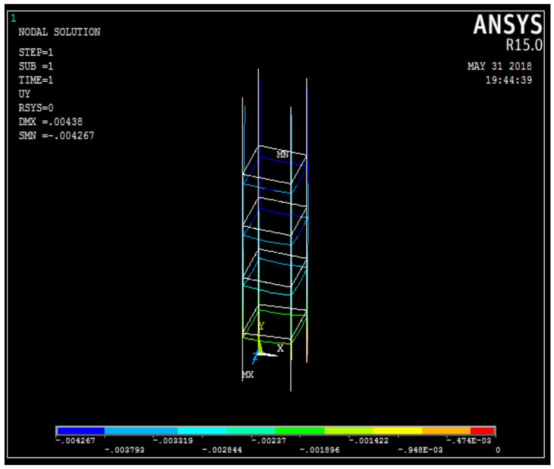

Figure 4: Displacement Solution for TLD

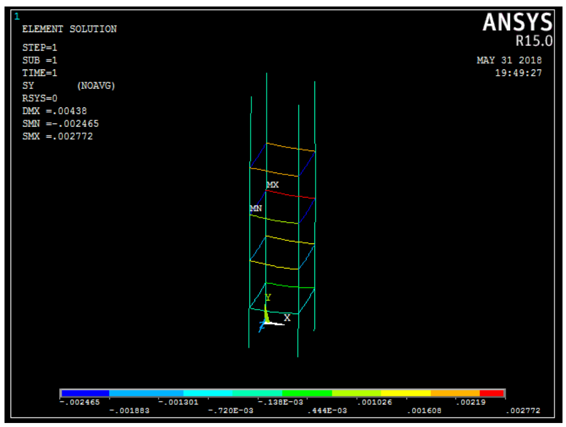

Figure 5: Stress solution for TLD

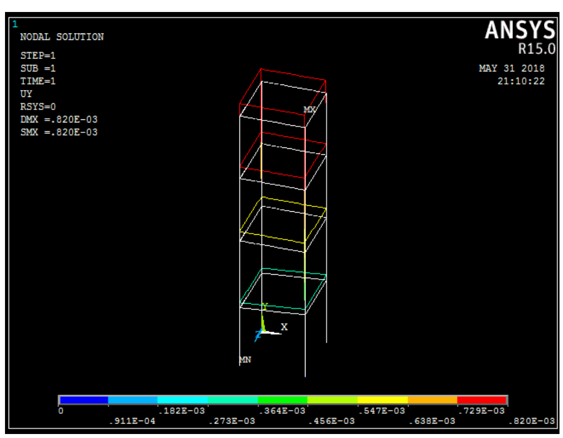

Figure 6: Displacement solution for TMD

Figure7:Stress solution for TMD

III. RESULTS AND DISCUSSION

In this investigation are considered to study the vibrant behaviour of assembly with TLD and TMD when exposed to harmonic base gesturespecified to the vibration table apparatus. From this work, the significance of displacement and acceleration of the assembly with and without TLD are measured. The result of experimental study has been plotted with water depth ratio and movement as frequency ratio. In table 2 the various liquid- depth ratio including external frequency and excitation occurrence ratio is given from experimentally.

Table 2: TLD experimental cases

Model	Water-depth ratio(Δ)	External frequency in Hz	Excitation frequency ratio (ω/ω_s)
TLD	0.15, 0.25, 0.35, 0.45, 0.5	4.0, 4.3, 4.6, 4.9, 5.2, 5.5, 5.8, 6.1	0.82, 0.88, 0.94, 1.0, 1.06, 1.12, 1.18, 1.24

In table 3 the different values of displacement are given with mass damper and without mass damper with its dynamic magnification factor.

Table 3: TMD experimental cases

Input frequency	Maximum Displacement(mm)		Dynamic Magnification Factor
	TMD	No TMD	
1.5	0.8	1.2	0.67
1.8	2	5.6	0.36
2.1	10.8	9.8	1.10
2.4	6.8	7.2	0.94
2.7	6.4	6.4	1.00

Effect of water depth ratio on structural response

Several water depth ratios, which is water depth (h) to tank length (L), varying from 0.15 to 0.5 are considered and maximum structural response has been specified in figure 8. It is also observed that displacement decreases with the increase in water- depth ratio. Similarly curves for acceleration vs. excitation occurrence ration are also plotted as

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specified in figure 9. The variation of displacement with excitation occurrence ratio for tuned mass damper system for structure is specified in figure 10.

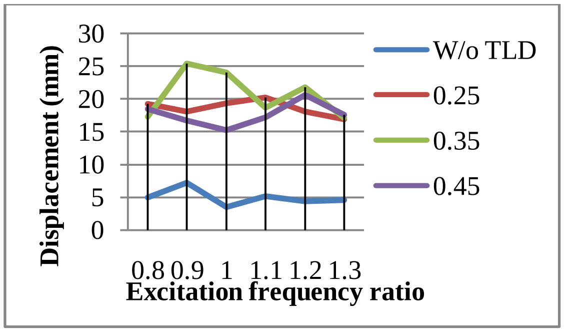

Figure 8: Variation of displacement with excitation frequency ratio for TLD

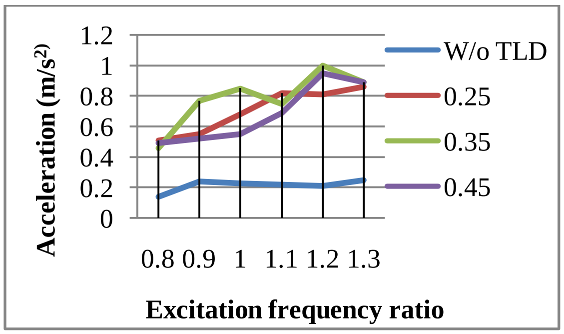

Figure 9: Variation of acceleration with excitation frequency ratio for TLD

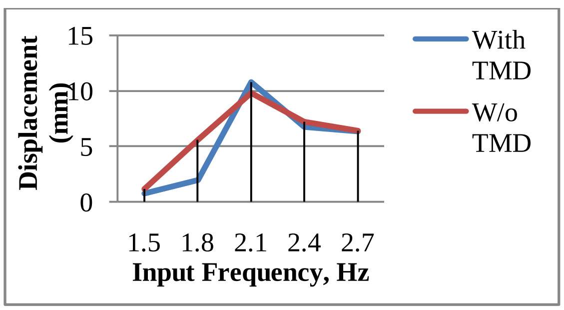

Figure 10: Variation of displacement with excitation frequency ratio for TMD

IV. CONCLUSIONS

In this paper, determination of structural replyunderneathvibrant loading by using liquid damper and mass damper are described. Based on this study following conclusions could be made;

1. The worth of displacement is decreases with increasing in water depth ratio.
2. Several excitation occurrence ratios varying from 0.8 to 1.3 are considered during this study. The performance of TLD is observed to be effective for reducing the structure response when excitation frequency ratio is near to unity.
3. Different liquid depth ratios varying from 0.15 to 0.5 are taken to consideration to evaluate performance at resonant condition.
4. From experimental study, it is observed that after using TMD optimum reduction is occurring when frequency ratio is near to unity.

5. With increase in mass ratio in TMD the peak displacement is decreasing and at certain times it again increasing with rise in mass ratio.

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